Supplementary Information for

Ultra-fast green hydrogen production from municipal wastewater by an integrated forward osmosis-alkaline water electrolysis system

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Supplementary Note 1. Simulation of specific energy consumption at different KOH temperatures

The specific energy consumption (SEC) was calculated at temperatures of at 10°C, 20°C, 30°C, and 40°C. The first step in the SEC calculation was to determine the specific normalised current (J_i) at each temperature. J_i is defined as the current needed to equalise the water permeation rate through the membrane matches with the consumption rate of water being released in the form of hydrogen and oxygen gas in the electrolyser. J_i was calculated using Supplementary Equation 1 for each of the four temperatures.

$$J_{i} = \frac{i_{cell}}{S} = \left(\frac{K_{w}I}{d}RT\frac{2F}{FE N_{cell}V_{m}}\right)\Delta C = \Phi \cdot \Delta C \quad (1)$$

where Φ is the H₂ production potential (L A mol⁻¹ cm⁻²), K_w is membrane permeability coefficient (L atm⁻¹ s⁻¹ cm⁻¹), *I* is the van't Hoff factor, *d* is the membrane thickness, *R* is the ideal gas constant, *T* is the temperature, *F* is the Faraday's constant, N_{cell} is the number of cells constituting the electrolyser stack, and V_m is the molar volume of H₂O. The new J_i values were then used to determine the *i_{cell}* at each temperature by multiplying the J_i with the membrane surface area utilised (1 cm²). The theoretical H₂ production rate (r_{H_2}) was then determined using the calculated *i_{cell}* values in Equation 2.

$$Q_{AWE} = i_{cell} \frac{FE \cdot N_{cell} V_m}{2F} \quad (2)$$

The next step was to determine the voltage value that corresponded to the calculated i_{cel} at each temperature. This was done by consulting the experimental current-voltage (J-V) curve, as shown in Supplementary Table 1. The identified voltage values were then input into Equation 3, along with the other required parameters to calculate the SEC for each temperature.

$$SEC = \frac{i_{cell} \times U}{r_{H_2}}$$
 (3)

where U is the voltage (V) obtained from J-V curves in the previous sections.

Supplementary Note 2. Supply of the oxygen produced from FOWS_{AWE} integrated systems to wastewater treatment plants

Supplying pure oxygen can substantially improve the efficiency of aerobic biological processes in wastewater treatment by raising dissolved oxygen levels and oxygen transfer rates, leading to higher treatment performance and reduced hydraulic residence times^{1–3}. The oxygen demand for the aerobic process in a wastewater treatment plant can be estimated by multiplying the average influent wastewater flow rate by the average chemical oxygen demand (COD) of the wastewater, as showed in Equation 4.

$$O_2 \ demand = Q \left(\frac{m^3}{day}\right) \times COD\left(\frac{kg}{m^3}\right) \quad (4)$$

where Q is the wastewater influent flow rate. As an example, a wastewater treatment plant receiving 50,000 m³ day⁻¹ of wastewater with a chemical oxygen demand of 400 mg L⁻¹ would theoretically require approximately 20,000 kg of oxygen per day. Based on the stoichiometric ratio between oxygen and hydrogen (2:1 in molar basis) during electrolysis, we can calculate the equivalent mass of hydrogen to produce that amount of oxygen. For the example requiring 20,000 kg day⁻¹ of oxygen, the equivalent mass of hydrogen would be 2,500 kg. This amount of hydrogen indicates the capacity needed for an electrolysis plant to meet the oxygen demand. Specifically, an electrolysis system with around 5-6 MW capacity would be capable of generating the required 2,500 kg of hydrogen per day to produce the needed 20,000 kg of oxygen daily.



Supplementary Fig. 1. Electrolyser performance with varied electrolytes. The current density-voltage (J-V) curves for the electrolyser using potassium hydroxide (KOH), sodium bicarbonate (NaHCO₃), and potassium diphosphate (K₄P₂O₇) electrolytes under the same conditions. (Conditions: [KOH] = [NaHCO₃] = [K₄P₂O₇] = 1 M). Source data are provided as a Source Data file.



Supplementary Fig. 2. Effect of draw solution on water flux. Comparison of water flux using NaCl and KOH as the draw solution and pure water as feed of the FO process. Error bars represent standard deviation from three independent replicates. (Conditions: [KOH] = [NaCl] = 1 M, flow rate = 190 ml min⁻¹). Source data are provided as a Source Data file.



Supplementary Fig. 3. Effect of KOH draw solution on FO membrane structure. The scanning electron microscopy (SEM) images of (a) the support layer of a pristine TFC-FO membrane, and (b) the support layer of a 5-cycle used TFC-FO membrane using 1 M KOH as the draw solution and simulated wastewater effluent as the feed solution.



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Supplementary Fig. 5. Influence of different wastewater effluent feed solutions. The rejection rates of the TFC-FO membrane for feeding different wastewater samples in presence of a wide range of impurities. (Conditions: [KOH] = 1 M, $S = 1 \text{ cm}^2$, operation duration = 5 h, $i_{cell} = 1.05 \text{ A}$ and 1.03 A for GD-WWE and NC-WWE, respectively, the species and content of impurity are detailed in Supplementary Table 4). Source data are provided as a Source Data file.



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Supplementary Fig. 7. Steady-state validation for the FOWSAWE system. The molar ratio of the H₂ generation rate to the water permeate rate (H₂/H₂O) using deionised water and real wastewater effluent as feed. Error bars represent standard deviation from three independent replicates. (Conditions: [KOH] = 1 M, membrane area = 1 cm²). Source data are provided as a Source Data file.



Supplementary Fig. 8. FO membrane structure after long-term operation. The SEM images showing **a**–**b**. the selective layer of a pristine TFC-FO membrane and the TFC-FO membrane after 168 h of operation, respectively; **c**–**d**. the support layer of a pristine TFC-FO membrane and the TFC-FO membrane after 168 h of operation, respectively. (Conditions: [KOH] = 1 M, $S = 1 \text{ cm}^2$, $i_{cell} = 0.90$ A for using HK-WWE2).



Supplementary Fig. 9. Hydrogen purity during long-term FOWS_{AWE} operation. H₂ purity measurements during 168 hours of continuous FOWS_{AWE} system operation. (Conditions: [KOH] = 1 M, $S = 1 \text{ cm}^2$, $i_{cell} = 0.90$ A for using HK-WWE2). Source data are provided as a Source Data file.



Supplementary Fig. 10. Influence of initial feed solution pH. The effect of initial wastewater effluent pH on the water flux. Measurements over time were carried out using GD-WWE with the initial pH adjusted to 6.5, 7.5, and 8.5. (Conditions: [KOH] = 1 M, $S = 1 \text{ cm}^2$, $i_{cell} = 1.05 \text{ A}$). Source data are provided as a Source Data file.



Supplementary Fig. 11. Process performance using different wastewater effluent samples. a. Water flux; b. voltage, c. concentration gradient (ΔC) between feed and draw solution, and d. H₂ flux of the integrated FOWS_{AWE} system when using different wastewater sources. (Conditions: [KOH] = 1 M, *S* = 1 cm², operation duration = 5 h, *i*_{cell} = 1.08 A, 1.05 A, and 1.03 A for HK-WWE2, GD-WWE, and NC-WWE, respectively). Source data are provided as a Source Data file.



Supplementary Fig. 12. FO membrane structure analysis. SEM images showing the selective layers of **a.** the pristine TFC-FO membrane, and **b.** the TFC-FO membrane after 5-h operation using GD-WWE.



Supplementary Fig. 13. Membrane performance comparison in the FOWS_{AWE} system. The comparison of the specific normalised current (J_i , i_{cell} S⁻¹), H₂ flux, and the product of the membrane permeability coefficient and van't Hoff factor (K_wI) between a TFC-FO membrane and a PTFE membrane using KOH as the operational solution of the FOWS_{AWE} integrated system. (Conditions: [KOH] = 1 M, membrane area = 1 cm²). Source data are provided as a Source Data file.



Supplementary Fig. 14. Electrolyser performance with different temperatures. The current density-voltage (J-V) curve performances for the FOWS_{AWE} integrated system under 10°C, 20°C, 30°C, and 40°C. Source data are provided as a Source Data file.



Supplementary Fig. 15. The independent FO module for testing KOH as the draw solution of the FO process. a. Experimental process design; and b. the detailed setup of the FO cell.



(b)

(a)



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Supplementary Fig. 16. The independent AWE module. The module consists in a 5-cell alkaline stack, an electrolyte supply tank, and a gas separator for collecting H_2 and O_2 for testing electrolysis performance. **a.** Experimental process design, and **b.** the detailed design of the gas separator.



(b)



Supplementary Figure 17. Integrating the modules of FO and AWE. a. Experimental process design, and **b.** experimental setups of the FOWS_{AWE} integrated systems.

(a)

Temperature	J_i (A m ⁻²)	Voltage (V)
10°C	0.91	1.85
20°C	0.94	1.80
30°C	0.98	1.72
40°C	1.01	1.65

Supplementary Table 1. The specific normalised current (Ji) and the resulting voltage obtained from the J-V curve to simulate SEC at different temperatures.

Reference	Process	kWh m ⁻³
(McGovern and Lienhard V, 2014)	Two-pass reverse osmosis	3.48 ¹
(McGovern and Lienhard V 2014)	Forward osmosis with draw	
(MCOOVERN and Elemnard V, 2014)	solute regeneration	5.56
(McGovern and Lienhard V, 2014)	Forward osmosis without draw	0 10 ³
	solute regeneration	0.10

Supplementary Table 2. Comparison of energy consumption for producing 1 m³ of water.

¹Assuming 47% efficiency for the first RO pass⁴.

²Assuming 47% efficiency for draw solute (NaCl) regeneration via RO⁴.

³Estimation of pumping power consumption⁴.

TFC-FO membrane	Element	Mass Norm (%)	Atom (%)	Mass Norm (%)
Pristine	С	82.08	85.92	3.80
	0	17.92	14.08	1.00
	C/O Ratio	4.58	6.10	/
After168-h use	С	80.00	84.18	3.59
	0	20.00	15.82	0.92
	C/O Ratio	4.00	5.32	/

Supplementary Table 3. EDX showing the C and O element and their ratio for the selective layer of a pristine TFC-FO membrane and the TFC-FO membrane after 168 h of operation.

Parameter	HK-WWE1 ⁽ⁱ⁾	HK-WWE2 ⁽ⁱⁱ⁾	GD-WWE ⁽ⁱⁱⁱ⁾	NC-WWE ^(iv)
Conductivity (mS cm ⁻¹)	N.M.*	0.89	4.87	1.44
TOC (mg L^{-1} as C)	5.23	18.21	20.53	44.36
pH	7.2	7.85	7.42	7.29
Color (PtCo)	N.M.*	46	86	37
Ammonia (mg L^{-1} as N)	1.09	0.48	0.59	0.12
Chloride (mg L ⁻¹)	150.2	1.27	9.42	2.56
Nitrite (mg L ⁻¹)	0.37	0.03	0.01	0.14
Bromide (mg L ⁻¹)	N.D.*	N.D.*	0.03	0.02
Nitrate (mg L ⁻¹)	4.43	0.05	0.37	0.28
Sulfate (mg L ⁻¹)	N.M.*	0.23	1.40	0.68
Phosphate (mg L ⁻¹)	N.M.*	0.19	0.38	0.22
Calcium (mg L ⁻¹)	N.M.*	29.24	74.25	101.14
Magnesium (mg L ⁻¹)	N.M.*	4.35	110.78	28.87
Zinc (mg L^{-1})	N.M.*	0.07	0.06	N.D.
Copper (mg L^{-1})	N.M.*	0.16	0.16	0.06
Iron (mg L^{-1})	N.M.*	0.10	0.09	0.05
Aluminium (mg L ⁻¹)	N.M.*	0.18	0.15	0.07

Supplementary Table 4. Quality of the collected municipal wastewater effluents samples.

*N.D.: not detected

*N.M.: not measured

*(i) HK-WWE1: wastewater effluent collected from Hong Kong; (ii) HK-WWE2: wastewater effluent collected from Hong Kong; (iii) GD-WWE: wastewater effluent collected from Guangdong province; and (iv) NC-WWE: wastewater effluent collected from Northeast China.

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